

1815 Banks Drive Weatherford, TX 76087 (817) 599-5241 www.jagmetalsllc.com

STRUCTURAL DETAILS (SSD)

SHEETING AND TRIM DETAILS: (SSHD)

SSD1 - DETAILS

SSD2 - DETAILS

SSD3 - DETAILS

SSD4 - DETAILS

SSHD1 - DETAILS

SSHD2 - DETAILS

SSHD3 - DETAILS

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SA2 - GENERAL NOTES

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SA3 - REACTIONS

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SS5 - FRAME LINE A

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SSH1 - ROOF SHEETING

SSH2 - SHEETING FRAME LINE 1

SSH3 - SHEETING FRAME LINE 5

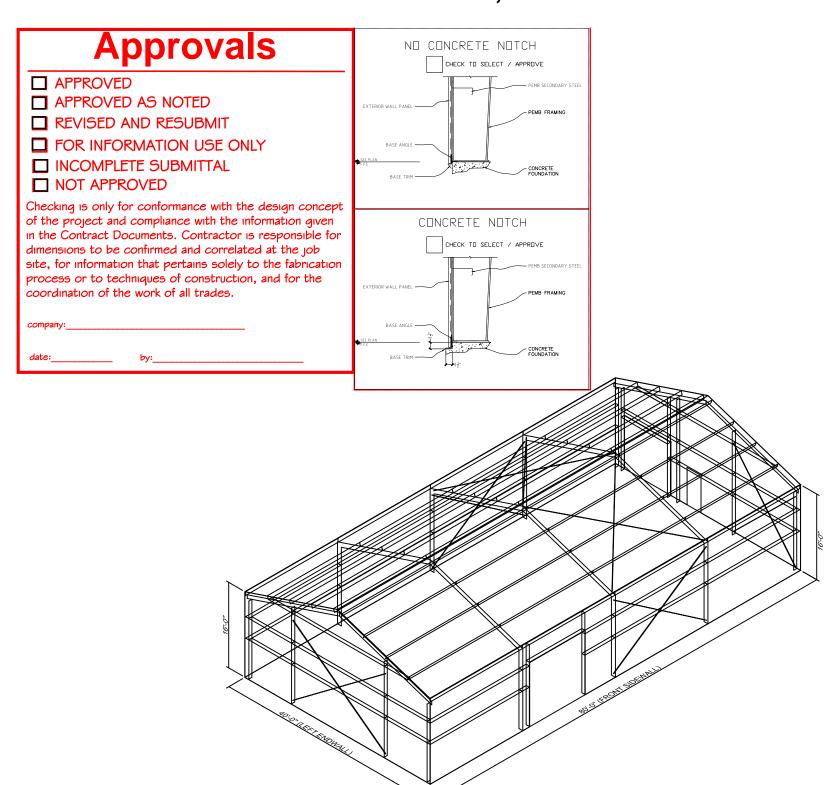
SSH4 - SHEETING FRAME LINE F SSH5 - SHEETING FRAME LINE A

# SHOP

PEMB STRUCTURAL ENGINEER: John Ketabi P.E

# PROJECT LOCATION- TBD BURLESON, TX 76028

PEMB DESIGNER: Loren J. Benedict





JOB ID	
6298F	R2.0
CUSTOMER	
Todd Dennis	3
PROJECT	
Shop	
J5p	
Burleson, TX	76028
DRAWING DATE:	. 0020
03/24	/2025
DRAWING STATU	S
FOR CONSTR	
FINAL DRAWINGS USED F	
NOT TO BE USED FOR ER	
FOR APPROV	
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REVISIONS	
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SCALING:	
NOT TO	SCALE

THE ENGINEER SIGNING AND SEALING THIS SET OF DRAWINGS IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT AND IS ONLY CERTIFYING THAT THE DESIGN OF THE BUILDING SYSTEM FURNISHED BY THE MANUFACTURER NAMED WITHIN SATISFY THE DESIGN REQUIREMENTS SPECIFIED BY THE CONTRACT DOCUMENTS IN ACCORDANCE WITH APPLICABLE STATE AND LOCAL BUILDING CODES AND STANDARDS OF PRACTICE AND DOES NOT EXTEND

BUILDING STRUCTURE DRAWN BY: CHECKED BY: ENG. BY: ΑK LJB

#### **GENERAL NOTES**

#### Engineering

The undersigned professional engineer is not the Engineer of the Record of the overall project. The seal is limited to the structural design of the framing and covering parts furnished and delivered by manufacture only. And exclude all Accessory items such as doors, windows, louvers, translucent panels, ventilators, foundations, masonry walls, mechanical equipment and the erection and inspection of the building. The buyer is responsible to verify specified loads are in compliance with the local regulatory authorities.

#### Contractor / End User Responsibilities

It is the responsibility of the contractor/end user to ensure that all project plans and specifications comply with the applicable requirements of any governing building authorities. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that the manufacturer or its design engineer is acting as the engineer of record for the project or design professional for a construction project.

The contractor/end user must secure all required approvals and permits from the appropriate agency. Approval of the drawings and calculations indicate that the manufacturer has correctly interpreted and applied the requirements of the contract, customer drawings, and specifications as were supplied to the manufacturer. (Sect. 4.2.1 AISC code of standard practices. 9th ed.)

Where discrepancies exist between the manufacturer's structural steel plans and the plans for other trades, the structural steel plans shall govern. (Sect. 3.3 AISC Code of Standard Practice 9th ed.)

Design considerations of any materials in the structure which are not furnished by the manufacturer are the responsibility of the contractor/end user and/or their engineers and not the manufacturer.

The contractor/end user is responsible for erection of steel and associated work in compliance with the manufacturer's "for construction" drawings. All bracings as shown and provided by the manufacturer for this building is required and shall be installed by the erector as a permanent part of the structure. Temporary supports, such as temporary guys, braces, false work, cribbing, or other elements required for the erection operation will be determined, furnished, and installed by the erector. These temporary supports shall secure the steel framing, or any partly assembled steel framing, against loads comparable in intensity to those for which the structure was designed, resulting from wind, seismic forces, and erection operations, but not the loads resulting from the performance of work by or the acts of others, nor such unpredictable loads as those due to tornado, explosion, or collision. (Sect. 7.9.1 AISC Code of Standard Practice, 9th ed.)

Warning: In no case should Galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosion effects on the aluminum zinc alloy coating when they are used in contact with galvalume steel panels. Even run-off from copper flashing, wiring, or tubing onto galvalume should be avoided as it can cause damage to the finish and void the warranty.

#### <u>Primer</u>

All structural steel to receive a rust inhibitive primer. This primer is not a final paint finish and is not intended for long term exposure to the elements.

#### **Approval Notes**

The following conditions apply if these drawings are used as approval drawings: It is imperative that any changes to these drawings be made in contrasting ink (preferably red ink), have all instances of change clearly indicated, and be legible and unambiguous. A signature and date are required on all pages.

The manufacturer reserves the right to re-submit drawings with additional details or changes as required to avoid fabrication errors. This may impact the delivery schedule. Approval of these drawings indicates conclusively that the manufacturer has correctly interpreted the contract documents and drawings, and further constitutes agreement that the building as shown on the manufacturer's plans, with indicated changes, represents the total of the materials to be supplied by the manufacturer to fulfill the contract agreement.

Any changes noted on the drawings not in conformance with the terms and requirements of the contract between the manufacturer and its customer are not legally binding unless, subsequently, specifically acknowledged and agreed to in writing by change order or separate documentation.

The manufacturer recognizes that "rubber stamps" are routinely used for indicating approval, disapproval, rejection, or mere review of the drawings submitted. However, the manufacturer does not accept changes or additions to contractual terms and conditions that may appear with use of a stamp or similar indication of approval, disapproval, etc. Such language applied to the manufacturer's drawings by the customer, architect, engineer, or any other party will be considered as unacceptable alternations to these drawing notes and will not alter the contractual rights and obligations existing between the manufacturer and its

Final detailing, fabrication, and delivery date of this project cannot be completed until signed approvals are returned to the manufacturer.

#### GENERAL DISCLAIMERS

#### Modifications to the Building System:

No modifications or alterations may be made to the building system or its components without obtaining prior written approval from JAG Metals, LLC's engineering department. This includes any changes to the design, materials, or construction methods that may deviate from the original specifications. Unauthorized modifications could significantly impact the structural integrity and overall performance of the building, and JAG Metals, LLC will not be responsible for any issues arising from such changes.

#### Risk of Unapproved Changes:

Any unauthorized changes made to the building system could compromise its structural integrity, potentially leading to an unsafe building design that endangers the safety of the occupants, the public, and the surrounding environment. Such alterations could also void warranties, insurance, and violate local building codes and regulations. JAG Metals, LLC strongly advises against making changes without proper engineering approval to ensure the building remains safe, compliant, and structurally sound

#### Anchor Bolt Responsibility:

The anchor bolts required for the building are the responsibility of others, and JAG Metals, LLC does not provide specifications regarding the type, length, or embedment of these anchor bolts. These specifications must be determined by the engineer of record based on the specific project requirements and local conditions. JAG Metals, LLC assumes no liability for the design, installation, or performance of anchor bolts, as these are outside the scope of our pre-engineered metal building system.

#### **Erection and Inspection Responsibilities:**

JAG Metals, LLC does not assume any responsibility for the erection of the building, the field supervision of the construction process, or for any special inspections that may be required by local building authorities during the erection phase. This includes, but is not limited to, the inspection of high-strength bolts, field welds, and other critical components of the structure. The responsibility for coordinating these activities and covering the costs associated with special inspections lies with the Erector, Owner, Architect, or Engineer of Record. It is essential that these parties ensure all necessary inspections are conducted in accordance with local regulations to guarantee the safety and compliance of the building.

#### Safety Compliance:

The Builder/Contractor is solely responsible for ensuring that all construction activities are conducted in full compliance with relevant state, federal, and OSHA safety regulations. This includes ensuring the safety of all personnel on-site and the use of appropriate safety measures, equipment, and procedures throughout the construction process. The Builder/Contractor is also responsible for ensuring that all OSHA standards and other pertinent safety rules are adhered to, and that the proper safety training is provided for all workers involved in the construction and erection of the building. JAG Metals, LLC is not responsible for any safety-related issues that may arise during the construction phase.

#### **COLORS**

ROOF PANELS:	Need SMP Color
WALL PANELS:	Need SMP Color
BASE TRIM:	Need SMP Color
GABLE/EAVE/GUTTER TRIM:	Need SMP Color
DOWNSPOUTS:	Need SMP Color
FRAMED OPENING TRIM:	Need SMP Color
CORNER TRIM:	Need SMP Color
LINER/SOFFIT PANEL:	
LINER/SOFFIT TRIM:	

PRIMARY FRAMING: Red Oxide SECONDARY FRAMING: Red Oxide

#### SERVICEABILITY (DEFLECTIONS)

- '	
ENDWALL COLUMN WIND:	180
ENDWALL RAFTER LIVE:	180
ENDWALL RAFTER WIND:	180
WALL GIRT WIND:	90
WALL PANEL WIND:	60
ROOF PURLIN LIVE:	180
ROOF PURLIN WIND:	150
ROOF PANEL LIVE:	60
ROOF PANEL WIND:	60
RIGID FRAME HORIZONTAL DRIFT:	60
RIGID FRAME VERTICAL:	180
RIGID FRAME SEISMIC:	50
RIGID FRAME CRANE:	100
WIND BENT HORIZONTAL DRIFT:	60
WIND BENT SEISMIC:	50
MEZZANINE DEAD + LIVE:	
MEZZANINE LIVE ONLY:	
CRANE RUNWAY HORIZONTAL:	
CRANE RUNWAY VERTICAL:	
OTHER:	N/A

#### MATERIAL NOTES

1.	MATERIALS:	MINIMUM	I YIELD:	
	HOT ROLLED BAR	Fy =	50.00	ksi MIN.
	STRUCTURAL STEEL SHEET	Fy =	50.00	ksi MIN.
	STRUCTURAL STEEL PLATE	Fy =	50.00	ksi MIN.
	COLD FORMED SHAPES	Fy =	57.00	ksi MIN.
	WALL SHEETING	Fy =	60.00	ksi MIN.
	ROOF SHEETING	Fy =	60.00	ksi MIN.
	BOLTS	A307 & A	325	
	THE METAL BUILDING MANUFACTURES	RESERV	ES THE RIGH	TT TO
	SUBSTITUTE THE ABOVE MATERIALS V	VITH FOU	AL OR BETTE	R MATERIA

2. BOLT TIGHTENING REQUIREMENTS:

ALL HIGH STRENGTH BOLTS ARE A325 UNLESS NOTED OTHERWISE. HIGH STRENGTH BOLTS SHALL BE TIGHTENED BY THE TURN OF THE NUT METHOD IN ACCORDANCE WITH THE LATEST EDITION OF AISC. "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". A325 BOLTS SHALL BE INSTALLED WITH OUT WASHERS WHEN TIGHTENED BY THE "TURN OF THE NUT" METHOD. ALL BOLTED CONNECTIONS, FOR SHEARDBEARING CONNECTION TYPES WITH BOLT THREADS EXCLUDED FROM THE SHEAR PLANE SHALL BE SNUG TIGHTON! Y

#### ACCESSORY SCHEDULE

1 - 3070M

( 1)3070M Walk Door

#### BASIS OF DESIGN

BUILDING DESCRIPTION	
WIDTH	40'-0"
LENGTH	80'-0"
BACK SIDEWALL HEIGHT	16'-0"
FRONT SIDEWALL HEIGHT	16'-0"
BACK ROOF SLOPE	4.0:12
FRONT ROOF SLOPE	4.0:12
THE CONTRACTOR IS TO LOADS COMPLY WITH TI THE LOCAL BUILDING DI	HE REQUIREMENT

CODES AND REFERENCES
BUILDING IBC 21
HOT ROLLED STEEL AISC16
COLD FORMED STEEL NAUS16

SEISMIC LOADS

EQUIVALENT LATERAL FORCE PROCEDURE
IMPORTANCE FACTOR
OCCUPANCY/RISK CAT.
MAPPED SPECTRAL RESPONSE
Ss 0.0489
Ss 0.0489
DESIGN SPECTRAL RESPONSE
Sds 0.0896
Sd1 0.0782
SITE CLASS d
DESIGN CATEGORY B
DESIGN BASE SHEAR 0.667\*le\*Sms\*W/R

LONGITUDINAL 0.60
TRANSVERSE 0.667"le"Sms"W/R
NOTE: THE STRUCTURAL STEEL SYSTEMS ARE NOT
SPECIFICALLY DESIGNED FOR SEISMIC RESISTANCE
SEISMIC RESPONSE COEF. (Cs)

 LEFT ENDWALL
 0.0299

 RIGHT ENDWALL
 0.0299

 FRONT SIDEWALL
 0.0299

 BACK SIDEWALL
 0.0299

 SEISMIC MODIFICATION FACT. (R)
 LEFT ENDWALL
 3.00

 RIGHT ENDWALL
 3.00
 3.00

 FRONT SIDEWALL
 3.00

 BACK SIDEWALL
 3.00

ADDITIONAL LOADS
SNOW DRIFT (psf)
MEZZANINE
DEAD (psf)
PARTITION DEAD (psf)
COLLATERAL ABOVE (psf)
COLLATERAL BELOW (psf)
LIVE (psf)
CRANE
CAPACITY (T)
SERVICE CLASS
BRIDGE WEIGHT (lbs)
HOIST/TROLLEY WEIGHT (lbs)

MAXIMUM WHEEL LOAD (lbs)

J A G M E T A L S L L C

6298R2.0

**Todd Dennis** 

PROJECT
Shop

Burleson, TX 76028

DRAWING DATE:
03/24/2025

DRAWING STATUS
FOR CONSTRUCTION
FOR CONSTRUCTION
FORL DRAWINGS USED FOR ERECTION PURPOSES
SCALED SET / PERMIT
NOT TO BE USED FOR ERECTION PURPOSES
REVISIONS

A

SCALING:
NOT TO SCALE

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ACCORDANCE WITH APPLICABLE STATE AND LOCAL BUILDING CODES AND STANDARDS OF PRACTICE AND DOES NOT EXTEND TO THE ERECTION OF THE BUILDING STRUCTURE.

DRAWN BY: TOHECKED BY: ENG. BY:

AK LJB JK

SEAL

HEET

SC2

#### **BACK SIDEWALL** (5) 80'-0" OUT-TO-OUT OF STEEL (2) **(4)** 20'-0" 20'-0" 20'-0" 20'-0" 1 1/2" 1 1/2" 1 1/2" X-Bracing A • • C ρ ⊕ ⊕ C • • C 1'-0" 1'-0" 1'-0" A [\* \*] 1'-0" E [\* \*] 10-0" (B) E 🛛 2'-0" В 40'-0" OUT-TO-OUT OF STEEL B × × 38'-8" (Hold This Dimension) RIGHT ENDWALL B × × $\overline{D}$ A [⊗ ⊗] • • C 1'-0" 1'-0" F 1 1/2" 1 1/2" X-Bracing 1 1/2" 1 1/2" 1'-6" 12'-0" 6'-6" 20'-0" 20'-0" 20'-0" 20'-0" (2) (3) (5)

FRONT SIDEWALL

### ANCHOR BOLT PLAN

NOTE: All Base Plates @ 100'-0" (U.N.)



6298R2.0
CUSTOMER
Todd Dennis
DDO IFOT
PROJECT
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Burleson, TX 76028
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NOT TO BE USED FOR ERECTION PURPOSES
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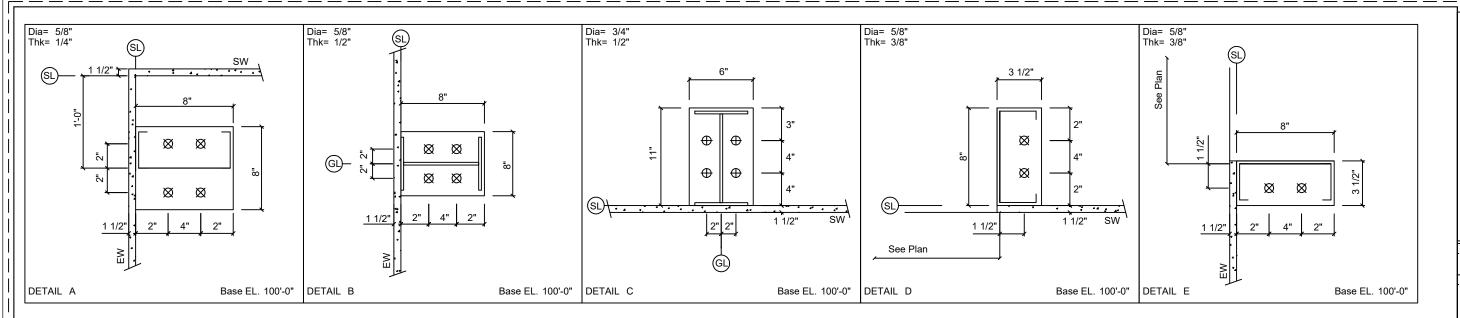
BUILE	ING STRUCT	URE.
DRAWN BY:	CHECKED BY:	ENG. BY:
AK	LJB	JK

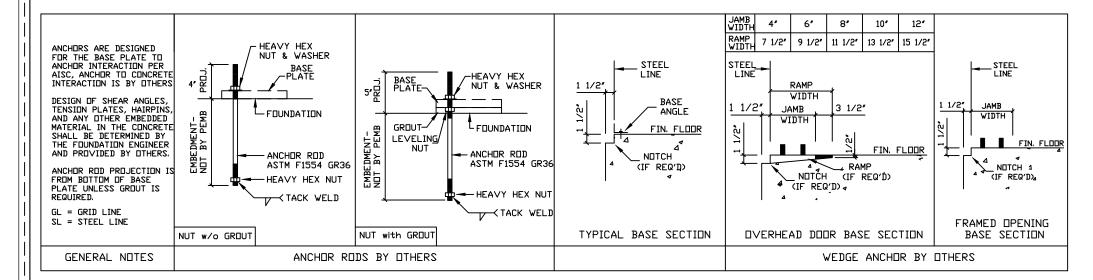
SEAL

SEAL

SHEE

SA1





### ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Туре	
<ul><li>⊗ 8</li><li>⊗ 32</li><li>⊕ 24</li></ul>	Jamb Endwall Frame	5/8" 5/8" 3/4"	A307 A307 A307	



# 6298R2.0

Todd Dennis

PROJECT

Shop

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AWING DATE: 03/24/2025

DRAWING STATUS

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SEALED SET / PERMIT

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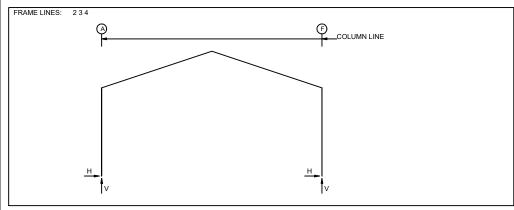
DRAWN BY: CHECKED BY: ENG. BY:

AK LJB JK

SEAL

HEET C

SA2



	RIGID	FRAM	E:	MAXIMU	IM REACT	IONS, A	NCHOR	BOLTS, &	BASE I	PLATES				
	Frm Line	Col Line	Load Id	Hmax H	lumn_Read V Vmax	ctions(k Load Id	) Hmin H	V Vmin	Bol Qty	t(in) Dia	Base Width	e_Plate(in) Length	Thick	Grout (in)
	2	Α	3 1	2.3 2.0	2.9 6.3	7 5	-2.8 -2.5	-2.2 -4.1	4	0.750	6.000	11.00	0.500	0.0
l	2	F	8 1	2.8 -2.0	-2.2 6.3	2 6	-2.3 2.5	2.9 -4.1	4	0.750	6.000	11.00	0.500	0.0

RIGIL	FRAM	E:	MAXIMU	IM REACT	IONS, A	ANCHOR	BOLTS, &	BASE	PLATES				
Frm Line	Col Line	Load Id	Hmax H	lumn_Read V Vmax	ctions(k Load Id	Hmin H	V Vmin	Bol Qty	It(in) Dia	Base Width	e_Plate(in) Length	Thick	Grout (in)
3*	Α	3 1	2.3 2.0	2.9 6.3	7 9	-2.8 0.8	-2.2 -4.4	4	0.750	6.000	11.00	0.500	0.0
3*	F	8 1	2.8 -2.0	-2.2 6.3	2 10	-2.3 -0.8	2.9 -4.4	4	0.750	6.000	11.00	0.500	0.0
3*	Frame li	nes:	3 4										

—W Loc	all — Line	- Col Line	——V Horz	Vind — Vert	- — Se Horz		- (lb/ Wind	ft) Seis
L EW	1	B.E	1.9	1.7	0.3	0.2		_
F_SW	Ė	3,4	3.1	2.3	0.9	0.6		
R_EW	5	F,D	1.9	2.7	0.3	0.4		
B_SW	Α	4,3	3.1	2.3	0.9	0.6		

RIGI	D FRAN	/IE:	BASIC CC	LUMN REA	ACTIONS (I	k)							
Frame	Column	Dea	ad	Collat	eral-	Live		Snc	)W	Wind	Left1-	-Wind	Right1-
Line	Line	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2	A	0.4	1.3	0.1	0.2	1.5	4.8	0.5	1.4	-4.6	-8.1	1.6	-5.0
2	F	-0.4	1.3	-0.1	0.2	-1.5	4.8	-0.5	1.4	-1.6	-5.0	4.6	-8.1
Frame	Column	Wind	Left2-	-Wind	Riaht2-	Wind	Long1-	Wind	Long2-	-Seism	ic Left	Seismi	c Right
Line	Line	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	- Vert
2	Α	-5.1	-5.0	1.2	-1.8	0.9	-6.4	0.0	-5.9	-0.1	-0.1	0.1	0.1
2	F	-1.2	-1.8	5.1	-5.0	0.0	-5.9	-0.9	-6.4	-0.1	0.1	0.1	-0.1
Frame	Column	F1UNE	SL L-	F1UNB	SL R-								
Line	Line	Horz	Vert	Horz	Vert								
2	Α	0.4	1.4	0.4	0.9								
2	F	-0.4	0.9	-0.4	1.4								
Frame	Column	Dea		Collat		Live		Sno		Wind	Left1-	-Wind	Right1-
Line	Line	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
3*	Α	0.4	1.3	0.1	0.2	1.5	4.8	0.5	1.4	-4.6	-8.1	1.6	-5.0
3*	F	-0.4	1.3	-0.1	0.2	-1.5	4.8	-0.5	1.4	-1.6	-5.0	4.6	-8.1
Frame	Column	Wind	Left2-	-Wind	Right2-	Wind	Long1-	Wind	Long2-	-Seism	ic Left	Seismi	c Right
Line	Line	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	_ Vert	Horz	Vert
3*	Α	-5.1	-5.0	1.2	-1.8	0.9	-8.7	0.0	-8.2	-0.1	-0.1	0.1	0.1
3*	F	-1.2	-1.8	5.1	-5.0	0.0	-8.2	-0.9	-8.7	-0.1	0.1	0.1	-0.1
Frame	Column	Seismi	c Long1	Seismic	Long2	F2UNB	SL L-	F2UNB	SL R-				
Line	Line	Horz	Vert	Horz	Vert	Horz	- Vert	Horz	Vert				
3*	Α	0.0	-0.6	0.0	0.6	0.4	1.4	0.4	0.9				
3*	F	0.0	-0.6	0.0	0.6	-0.4	0.9	-0.4	1.4				
3*	Frame line	s:	3 4										

END	WALL	CO	LUM	1N:	BASI	C COLUMI	N REAC	TIONS (I	<)									
Frm Line 1 1 1	Col Line A 0 B 0 E 0 F 0	.7 .7		1	Live Vert 0.4 3.7 3.7 0.4	Sno Ver 0.1 0.6 0.6 0.1	t 0 -1 0	.9 - .0 -		Wind_ Horz 0.0 0.0 1.9 0.0	Right1 Vert -0.9 -1.0 -6.0 -0.4			ert 0 0 1	Wind_ Horz .0 .0 .9	Right2 Vert -0.3 0.0 -4.9 0.2	Wind Press Horz 0.0 -2.7 -2.7 0.0	
Frm Line 1 1 1		.0		) - 7 -	Vert 1.3 2.7	-0.7 -: 0.0 -:	ong2 Vert 0.9 2.8 2.7	Seis Horz 0.0 -0.3 0.0 0.0		Seis Hor: 0.0 0.0 0.3 0.0	s_Right z Ve 0.0 0.2 -0.3 0.0	0	Seis Long Vert 0.0 0.0 0.0	E1U Horz 0.0 0.0 0.0 0.0	INB_SI 2 V 0.0 0.8 0.4 0.0	ert		
Frm Line 1 1 1		.0	3_SL_ Ve 0.0 0.4 0.8 0.0	R- rt														
Frm Line 5 5 5 5	Col Line F 0 D 0 C 0 A 0	.5		1	Live Vert 1.2 2.9 2.9 1.2	Sno Ver 0.2 0.5 0.5 0.2	t -1 0 0	.0 -	Vert 4.1 0.7 1.7	Wind_ Horz 0.0 1.9 0.0 0.0	Right1 Vert 1.7 -5.1 -3.5 -1.3		-3.3 0.0 -0.9	ert 0 1 0	Wind_ Horz .0 .9 .0	Right2 Vert 2.5 -4.3 -2.7 -0.5	Wind Press Horz 0.0 -2.5 -2.5 0.0	
Frm Line 5 5 5 5	Col Line F 0 D 2 C 2 A 0	.8 .8	0.0 0.7 0.0 0.0	7 - ) -	Vert	0.0 -	ong2 Vert 2.4 0.5 2.8 2.1	Seis Horz -0.3 0.0 0.0 0.0	Left Vert -0.4 0.4 0.0 0.0	Seis Hor: 0.0 0.3 0.0 0.0	s_Right z Ve 0.5 -0.5 0.0 0.0	0	Seis Long Vert 0.0 0.0 0.0	E2U Horz 0.0 0.0 0.0 0.0	INB_SI 2 V 0.2 0.7 0.3 0.1	ert		
Frm Line 5 5 5 5	Line Line Horz Vert 5 F 0.0 0.1 5 D 0.0 0.3 5 C 0.0 0.7																	
END	ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES																	
Frn Lin			Load Id	Hmax H	Column_R V Vma	teactions(k Load ax Id	) Hmin H	V Vmi		Bolt(in)	ia Wi		_Plate(in) Length	Thick	Gro (ir			
1	Α		9 1	0.0	-0.7 0.5	9	0.0	-0.7	4	0.625	5 4.0	000	8.000	0.250	0.0	)		
1	В		11 1	1.8 0.0	-3.2 4.4	12 11	-1.6 1.8	-1.3 -3.2	4	0.625	5 8.0	000	8.000	0.500	0.0	)		
1	E		13 1	1.8 0.0	-3.2 4.4	14 13	-1.6 1.8	-1.3 -3.2	4	0.625	5 8.0	000	8.000	0.500	0.0	)		
1	F		10 1	0.0	-0.7 0.5	10	0.0	-0.7	4	0.625	5 4.0	000	8.000	0.250	0.0	)		
5	F		5 4	0.0	-2.3 2.2	5	0.0	-2.3	4	0.625	5 4.0	000	8.000	0.250	0.0	)		
5	D		13 1	1.7 0.0	-2.7 3.5	14 13	-1.5 1.7	-2.1 -2.7	4			000	8.000	0.500	0.0			
5	С		13 1	1.7 0.0	-1.8 3.5	12 13	-1.5 1.7	-1.3 -1.8	4			000	8.000	0.500	0.0			
5	Α		10 1	0.0	-1.1 1.4	10	0.0	-1.1	4	0.625	5 4.0	000	8.000	0.250	0.0	)		

#### NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- 2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.

Building reactions are based on the following building data:

Width (ft) = 40.0

Length (ft) = 80.0

Eave Height (ft) = 8.0.0

Roof Slope (rise/12) = 4.00/4.00

Roof Dead Load

Wall Dead Load

Wall Dead Load Width
Length
Eave Height
Roof Slope (rise/
Roof Dead Load
Wall Dead Load
Left Endwall
Right Endwall
Roof Live Load
Collateral Load
Snow Load
Wind Code
Exposure
Closure
Internal Wind Coeff
Risk Category
Importance - Wind
Importance - Seismic
Seismic Coeff
Seismic Coeff
Sadding conditions are: (psf) = 2.0 (psf) = 20.0 (psf) = 12.0 (psf) = 0.5 (psf) = 3.5 (mph) = 107.0 =IBC 21 = C = Enclosed = -0.18, +0.18 = II - Normal = 1.00 = 1.00 = 8

=B (Sms) = 0.13

5. Loading conditions are:

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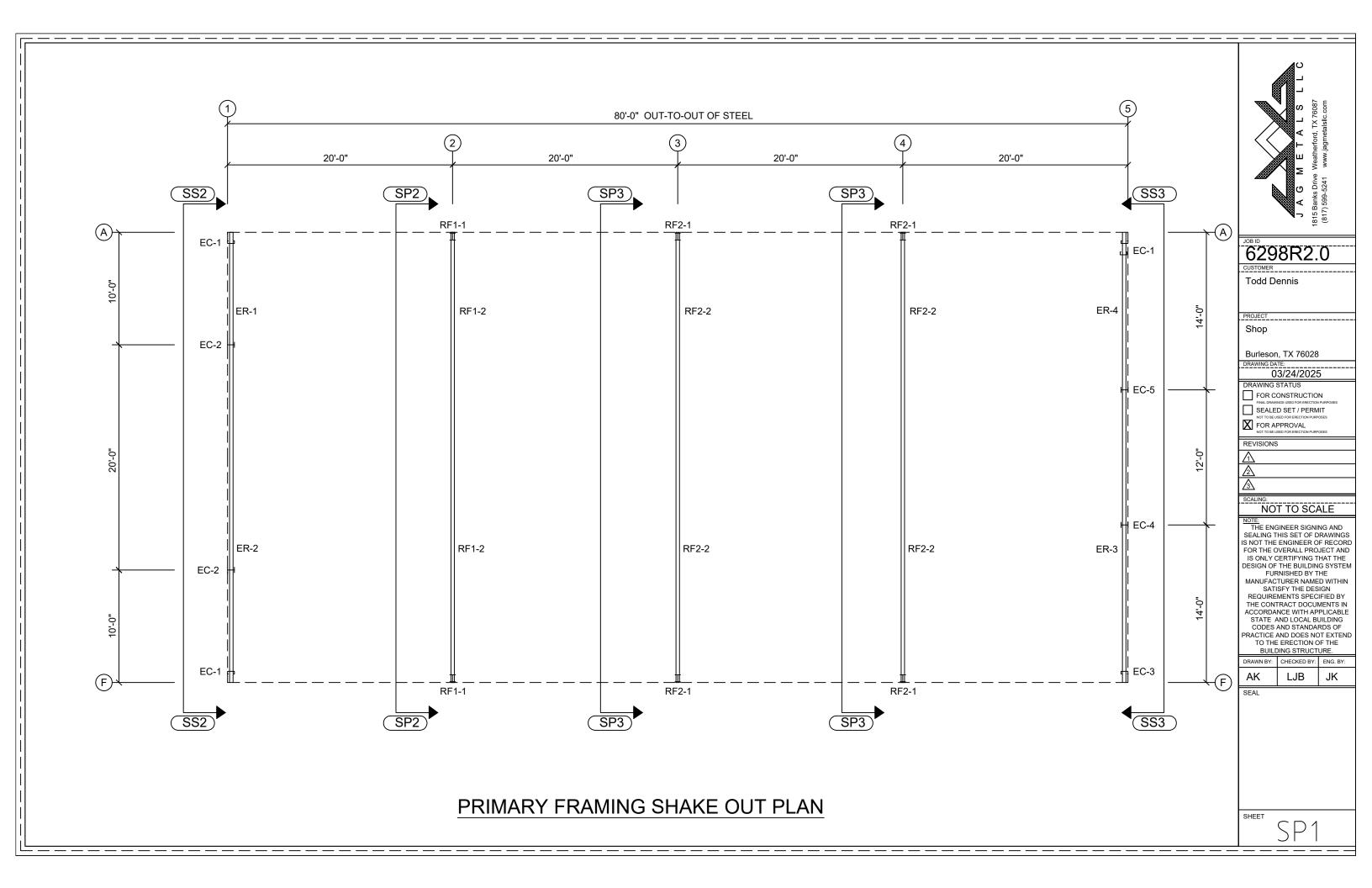
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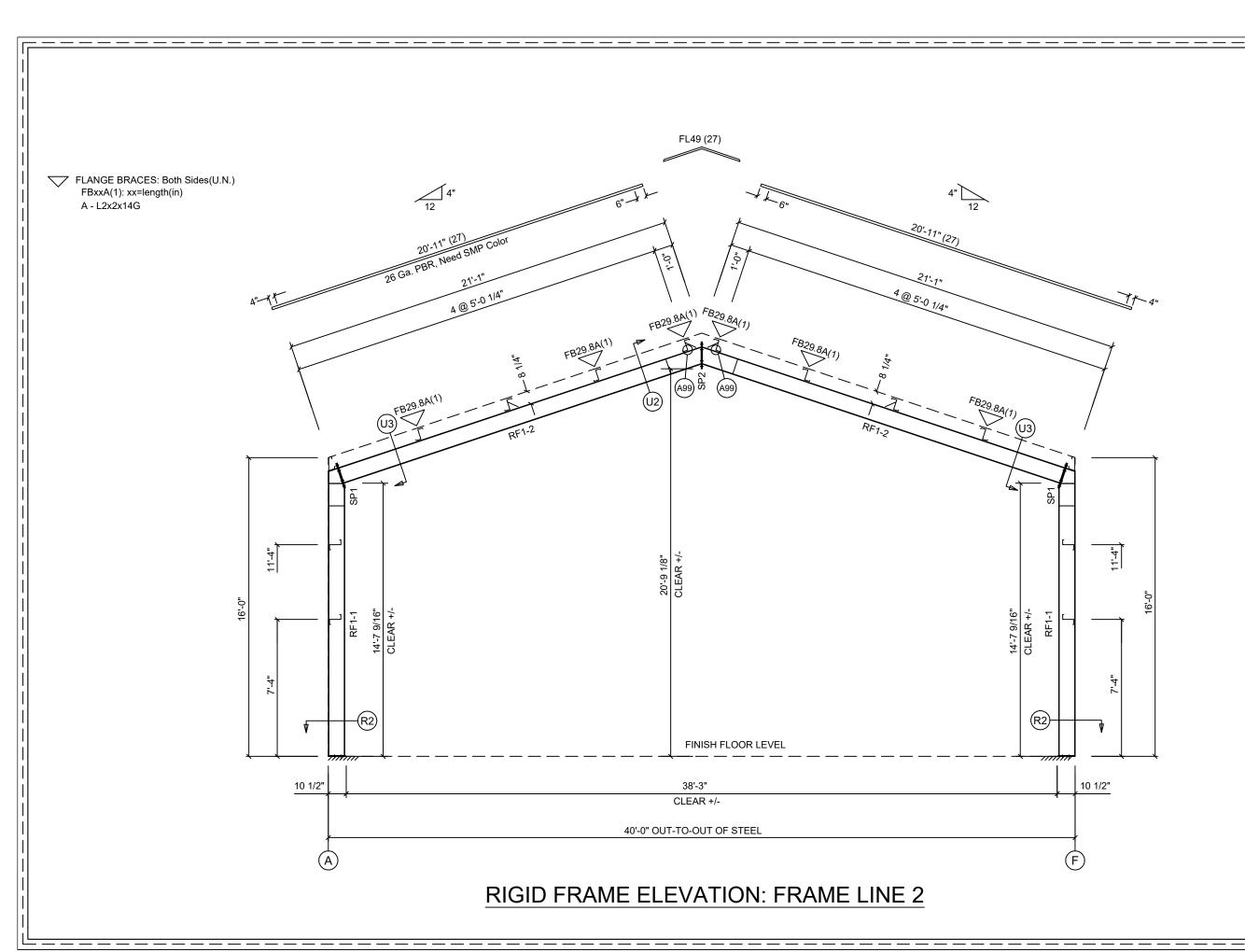
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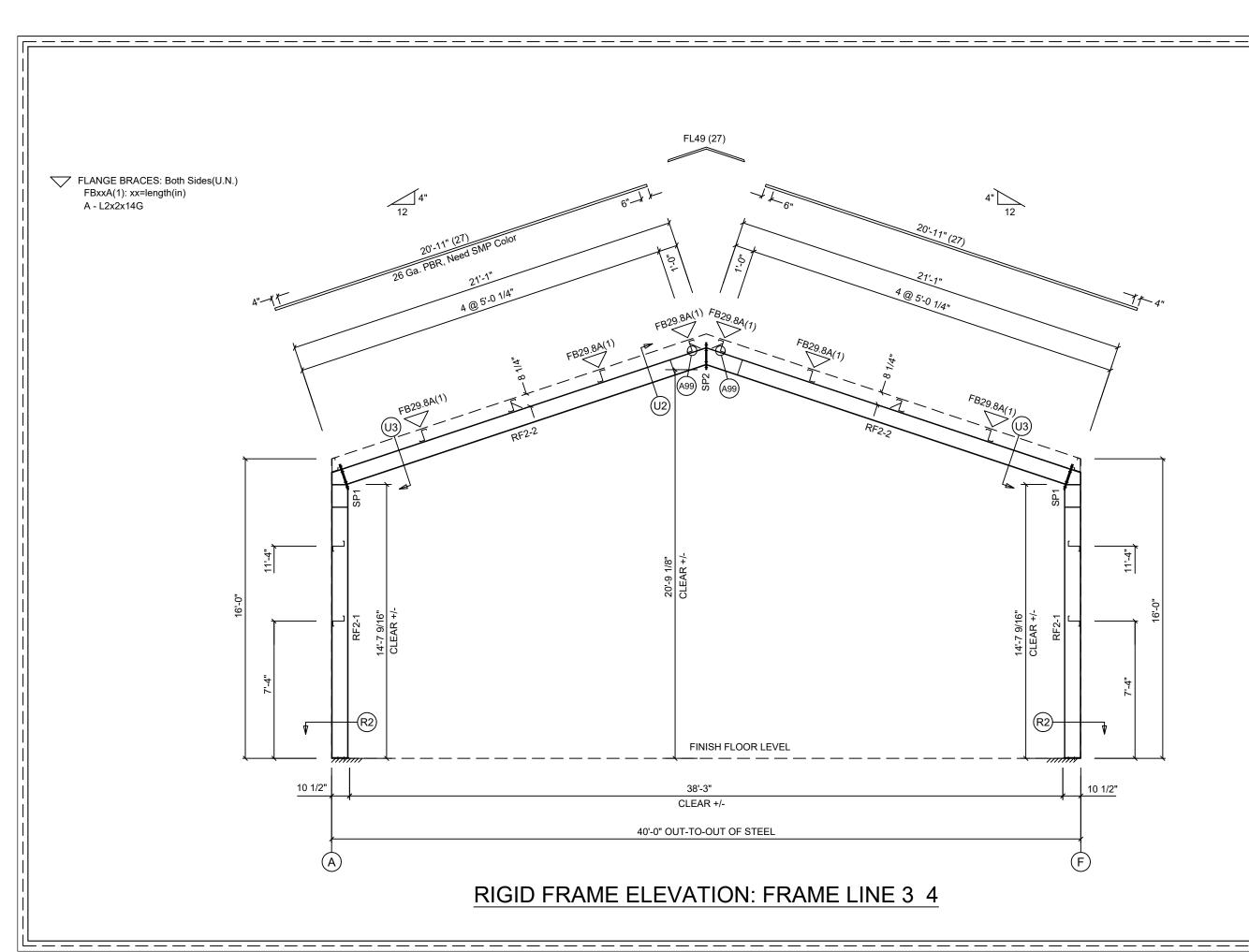
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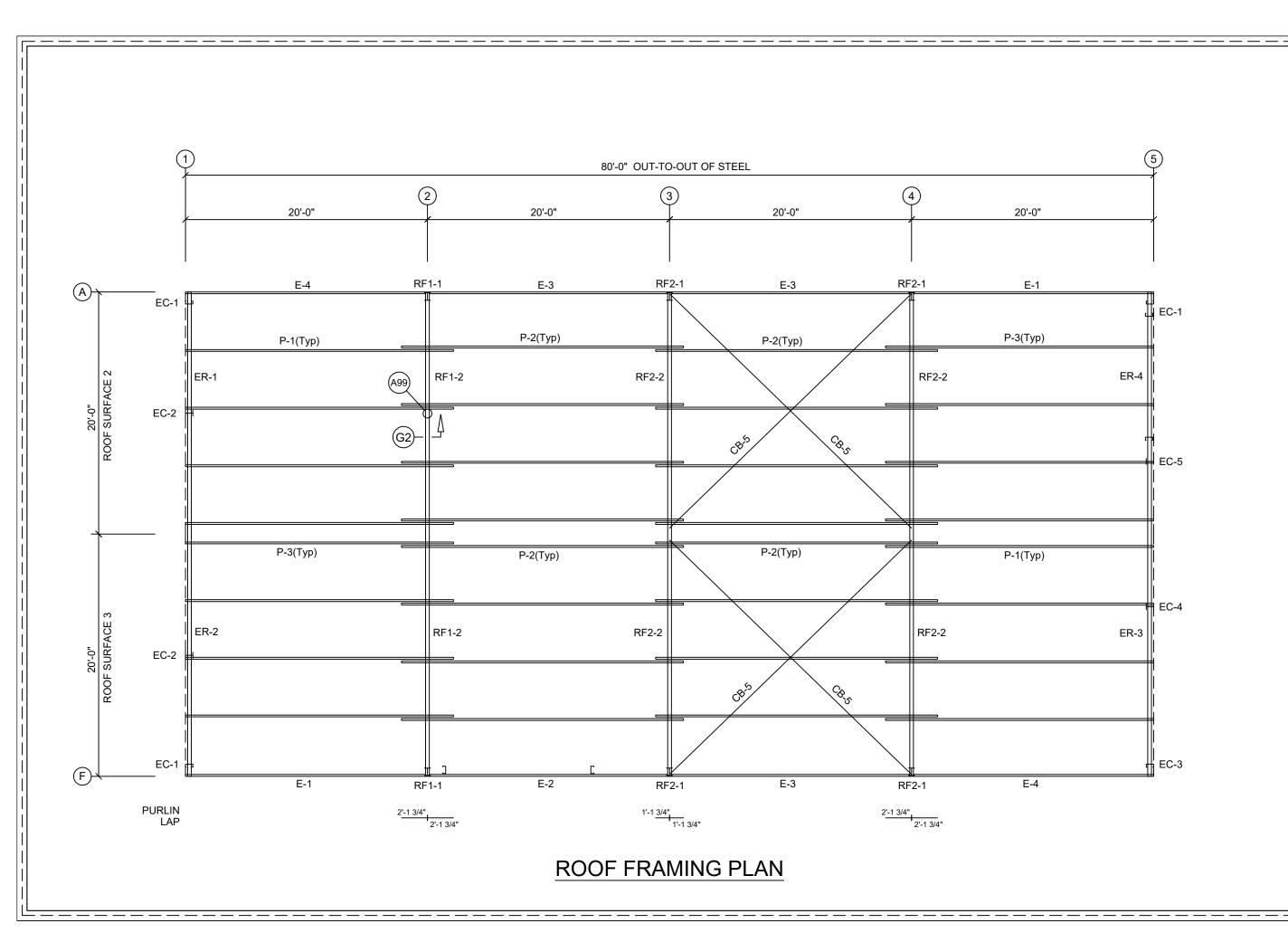
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J A G M E T A L S L L C (815 Banks Drive Weatherford, TX 78087 (817) 599-5241 www.jagmetalstlc.com

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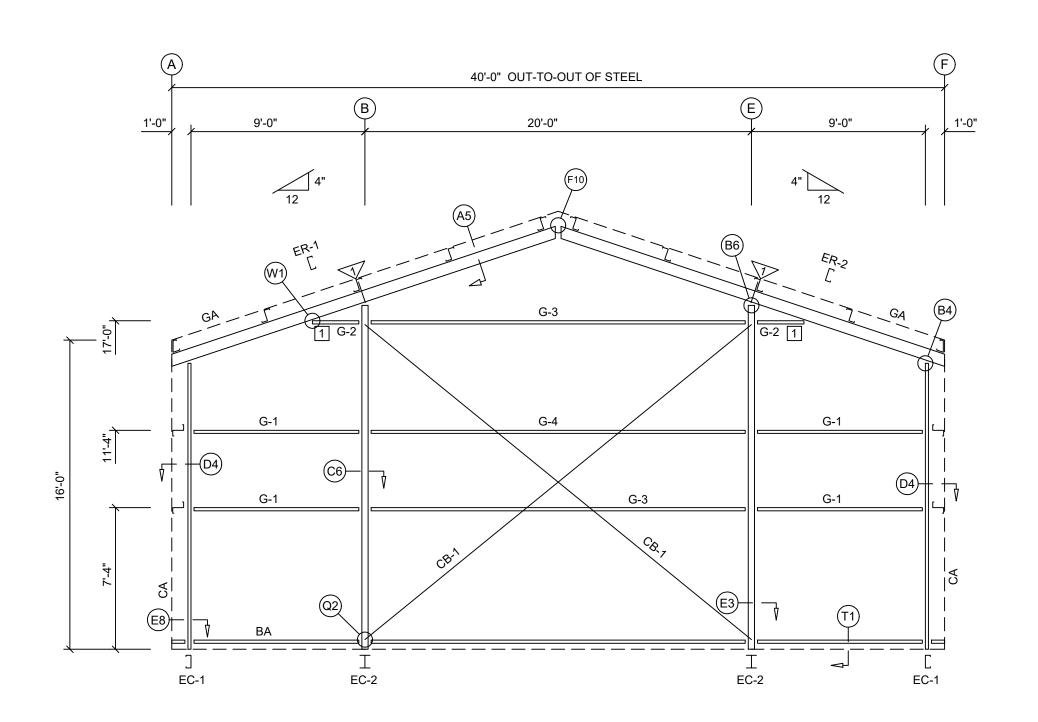
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SEAL

SHEE

SS1





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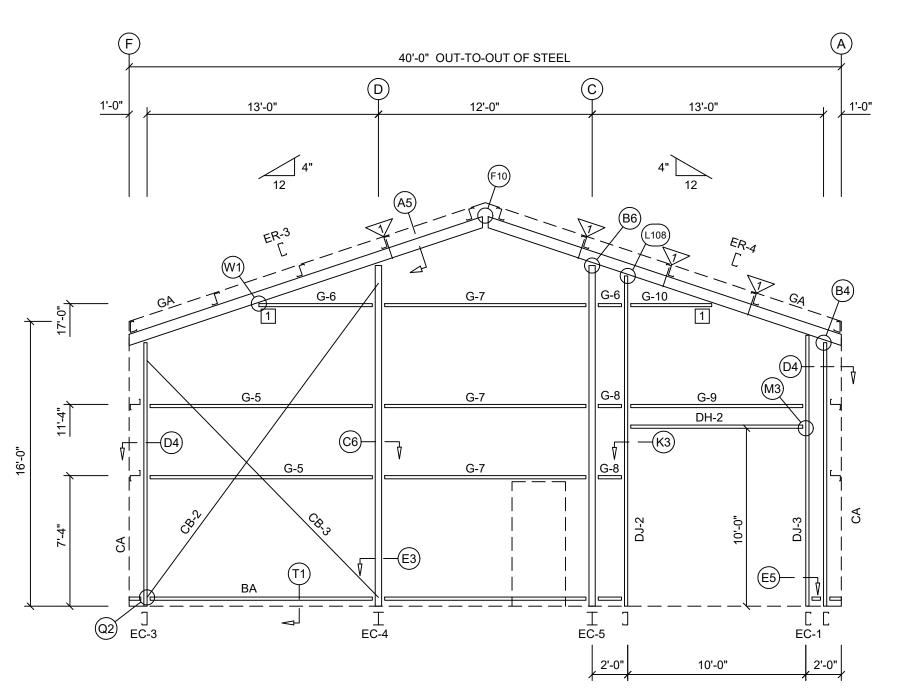
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SEAL

LEFT ENDWALL FRAMING: FRAME LINE 1



RIGHT ENDWALL FRAMING: FRAME LINE 5



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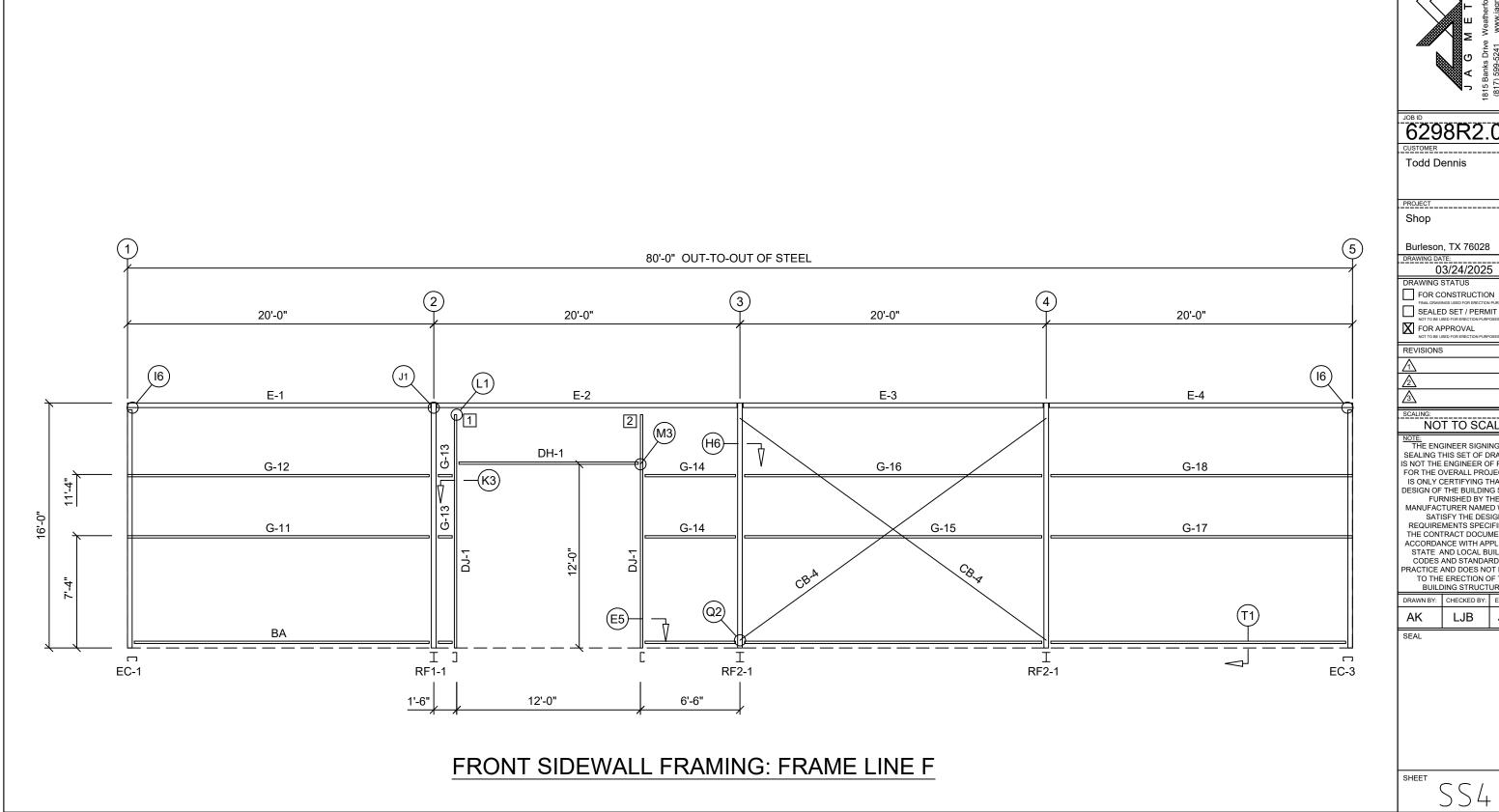
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BUILDING STRUCTURE.





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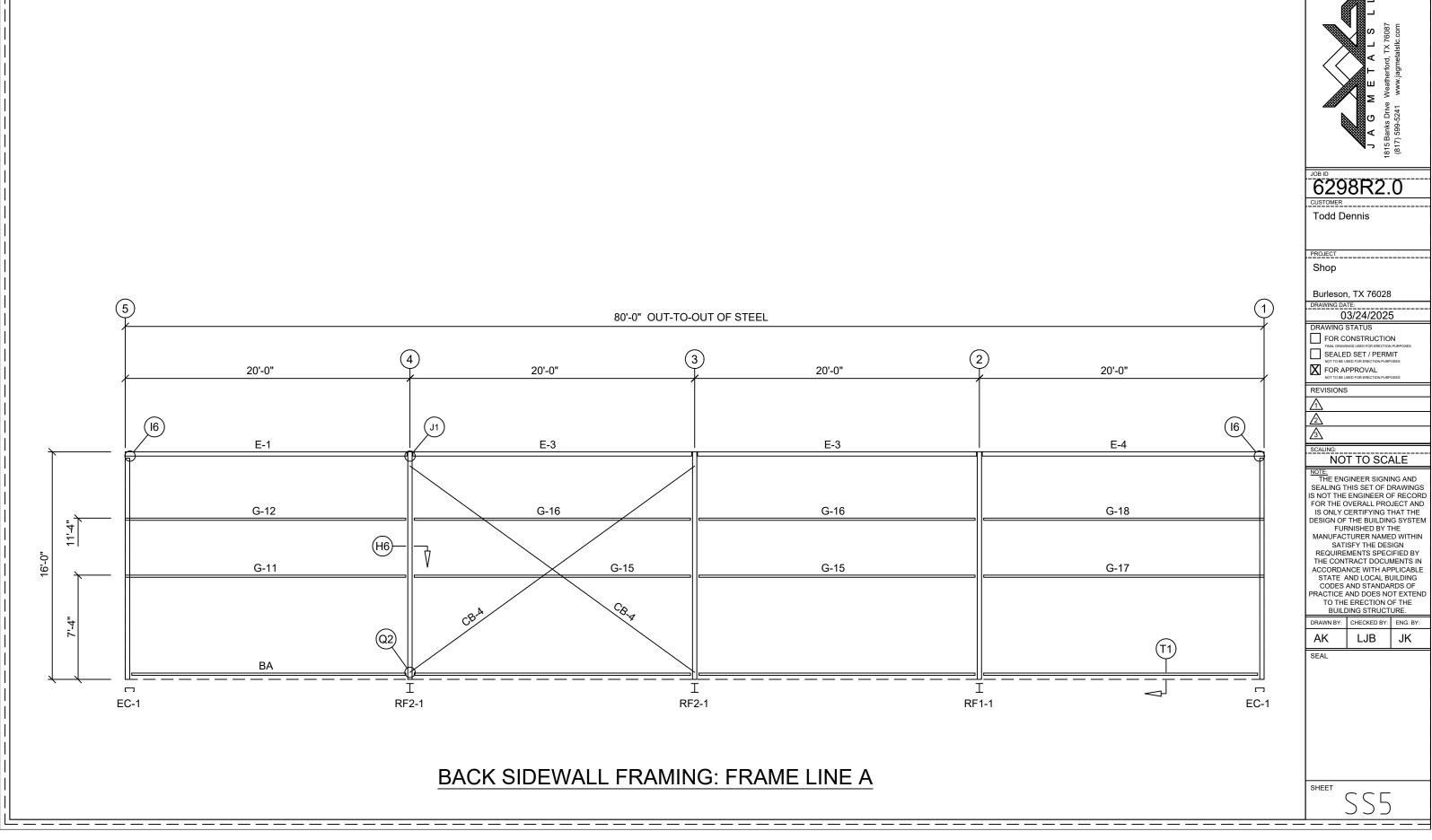
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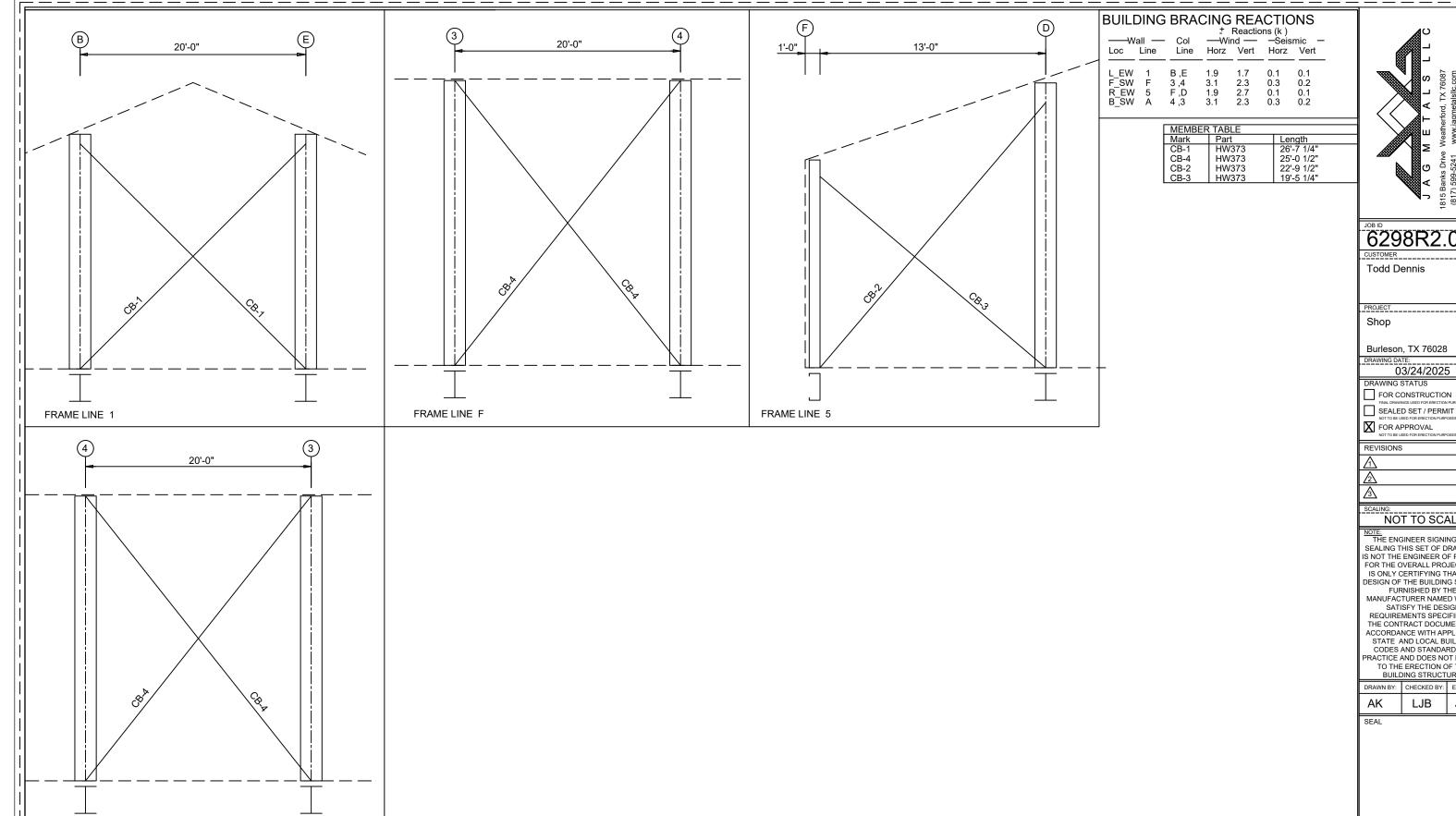
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ΑK LJB JK





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WIND FRAME LAYOUT

FRAME LINE A

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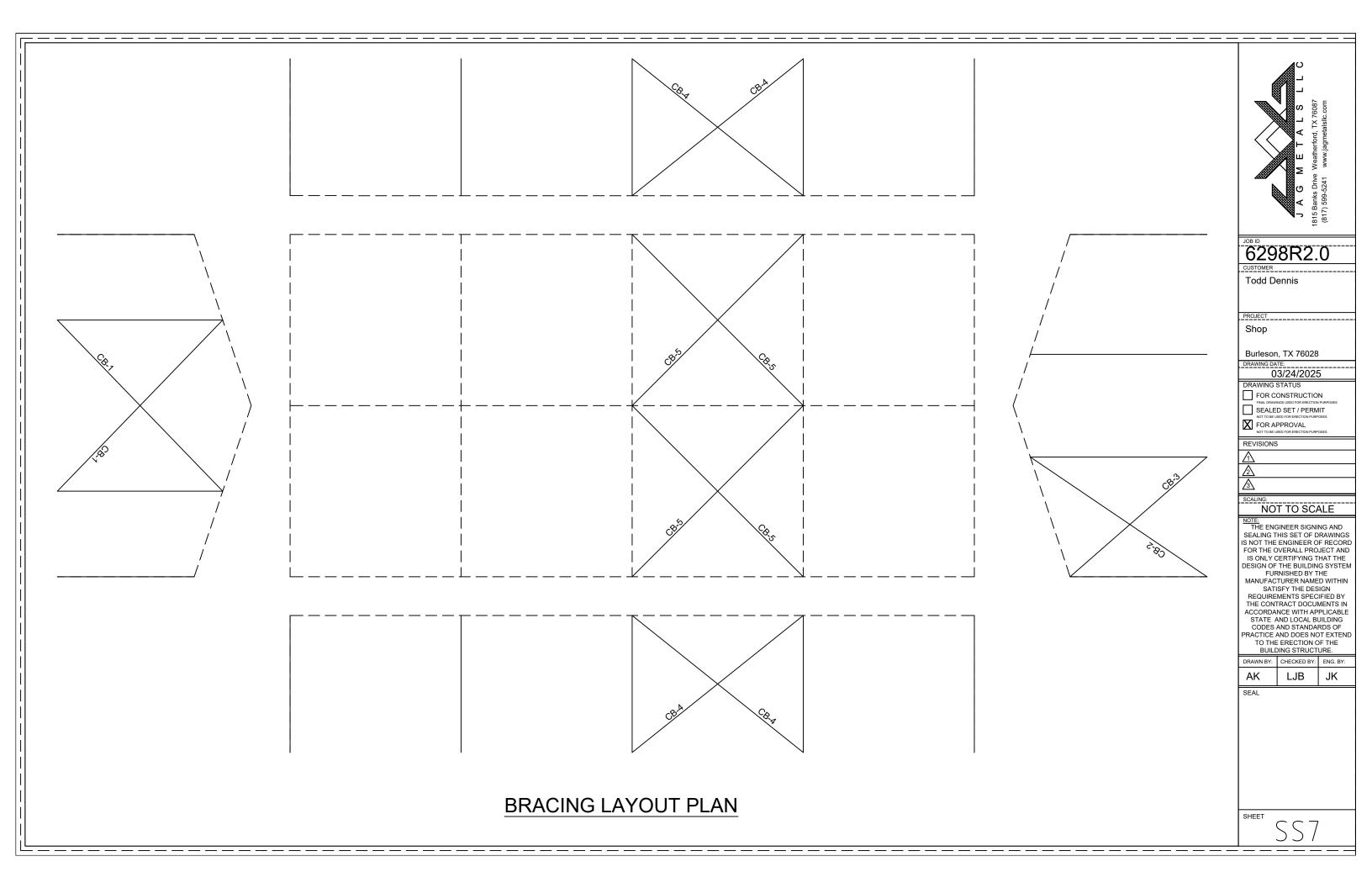
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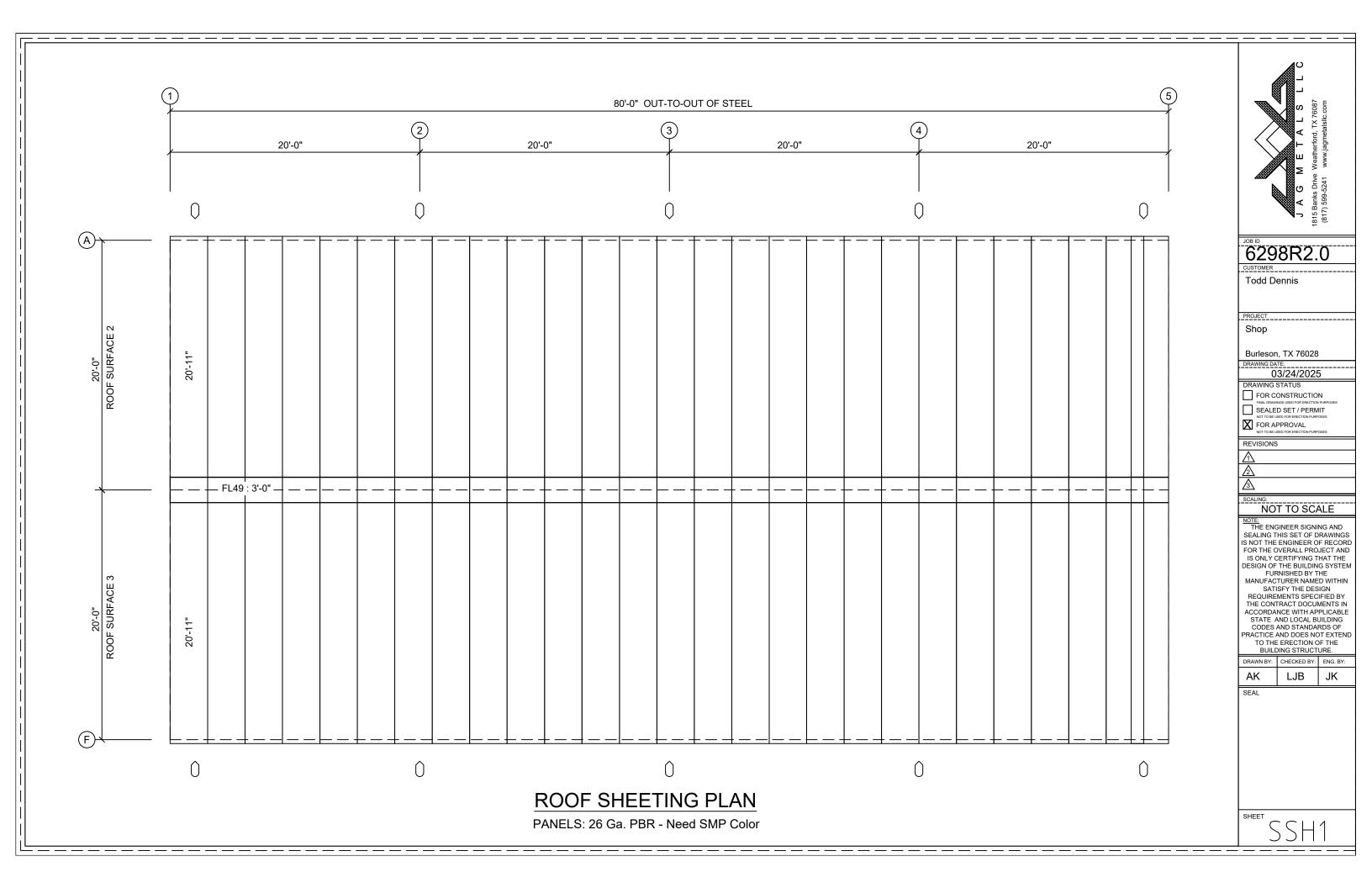
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TRIM TABLE (Trim laps 2" unless noted otherwise)								
QUAN	MARK	LENGTH	DETAIL					
2	FL16D	20'-2"	TRIM_103					
1	FL16	10'-2"	TRIM_103					
2	FL833	16'-0"	TRIM_13					
5	FL72	10'-2"	TRIM_1					
1	FL16B	1'-4"						



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AK LJB JK	DRAWN BY:	CHECKED BY:	ENG. BY
	AK	LJB	JK

SEAL

LEFT ENDWALL SHEETING & TRIM: FRAME LINE 1

FL72: 10'-2"

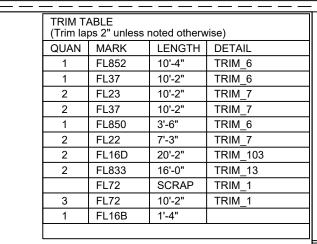
FL16B : 1'-4"

22'-8" 22'-8"

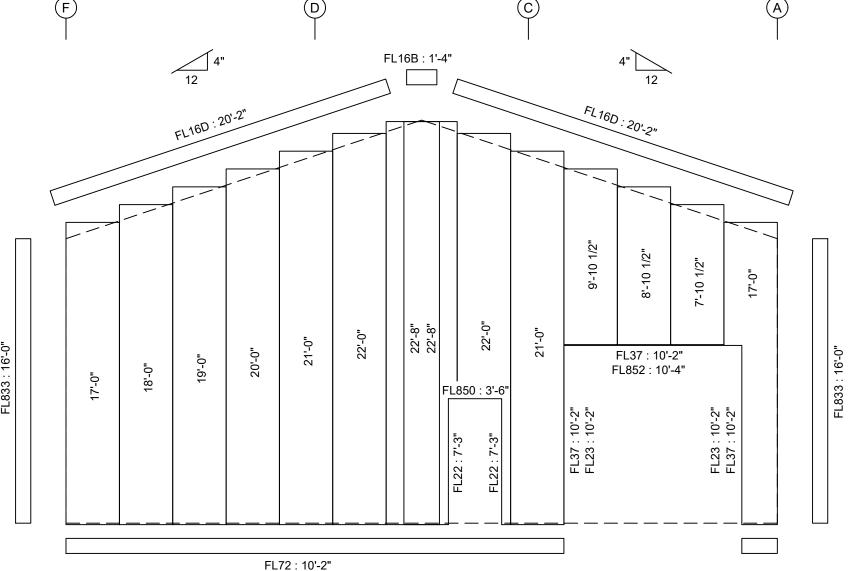
FL16:10'-2", FL16D:20'-2"

PANELS: 26 Ga. PBR - Need SMP Color

SSH2







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TO THE ERECTION OF THE BUILDING STRUCTURE.

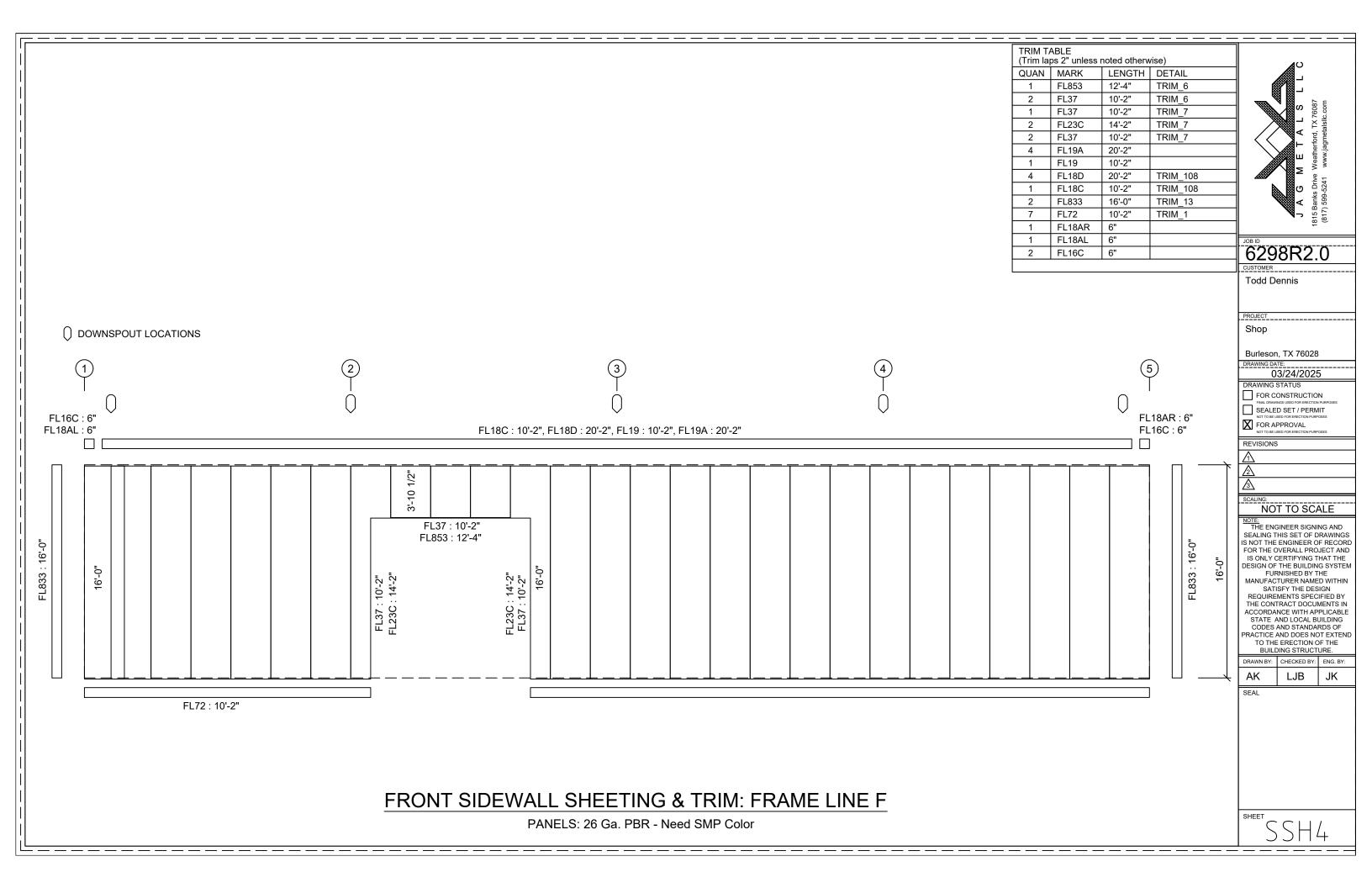
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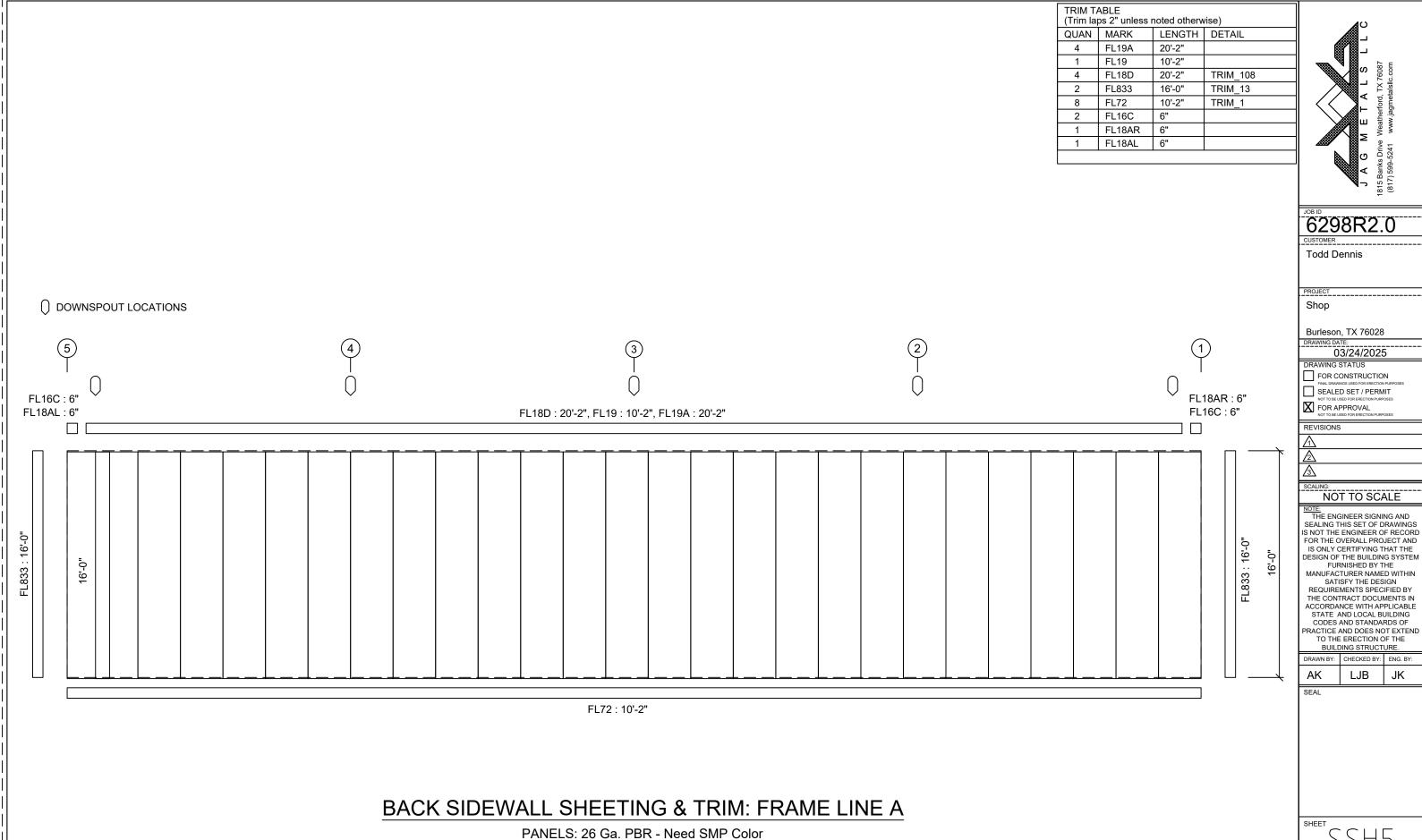
SHEET

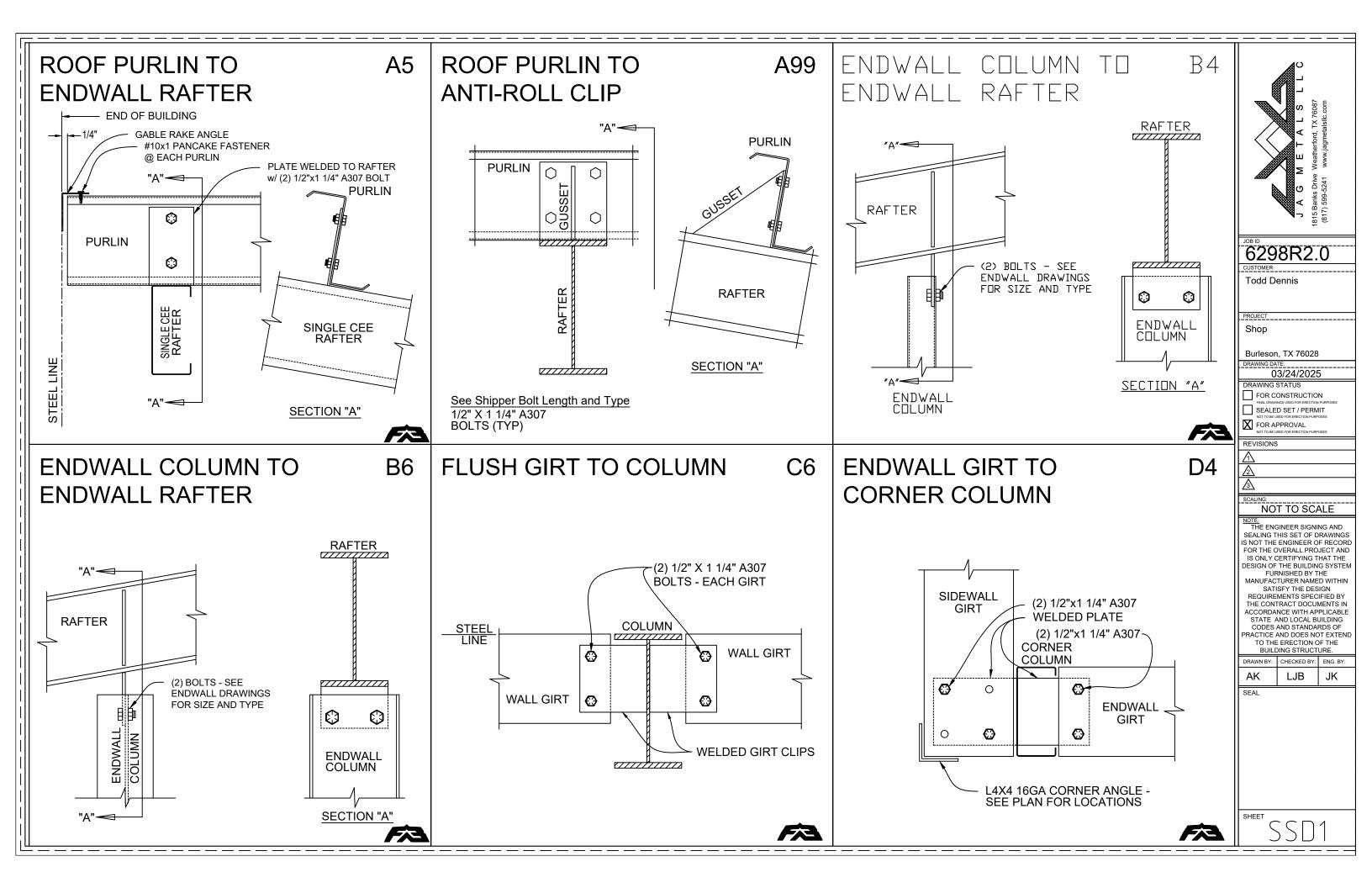
SSH3

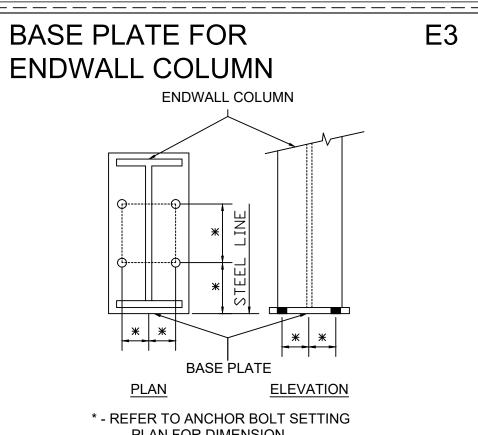
RIGHT ENDWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 26 Ga. PBR - Need SMP Color









**ENDWALL** 

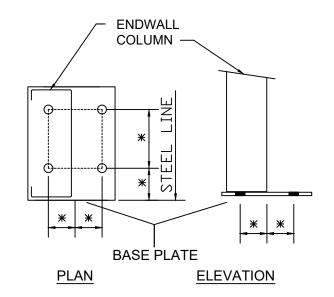
**RAFTER** 

SLICE PLATE CONNECTION

PLATE- SEE PLAN FOR

**LOCATIONS** 

### BASE PLATE FOR ENDWALL E8 **COLUMN OR DOOR JAMB**



\* - REFER TO ANCHOR BOLT SETTING PLAN FOR DIMENSION

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**(D)** 



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VING STATUS FOR CONSTRUCTION

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H6

WELDED CLIP

@ EACH END

€Э

€€

1/2"x1 1/4" A307 BOLT

**GIRT** 

(2) 1/2"x1 1/4"

A307 BOLT

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BUILDING STRUCTURE DRAWN BY: CHECKED BY: ENG. BY:

ΑK LJB

Erection Note: Flange Braces may be required one or both sides of Column. For requirements, and locations at Main Frame and Rigid Frame in Endwall

€€

# RIGID FRAME RAFTER

 $\circ$ 

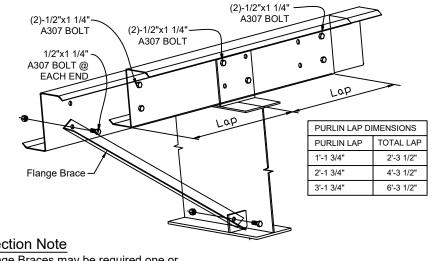
PLAN

**ENDWALL** 

COLUMN

BASE PLATE

**ELEVATION** 



**Erection Note** 

Flange Braces may be required one or both sides of Rafter.

For requirements, and locations at Main

Frame and Rigid Frame in Endwall refer to



**WALL GIRT TO** 

WELDED CLIP

@ EACH END

**GIRT** 

WELDED CLIP

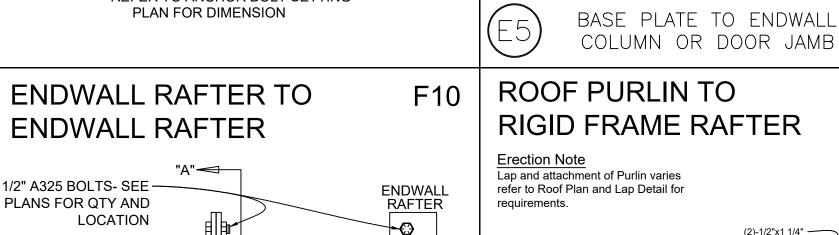
1/2"x1 1/4" A307 BOLT

(2) 1/2"x1 1/4"

**(** 

A307 BOLT

RIGID FRAME COLUMN

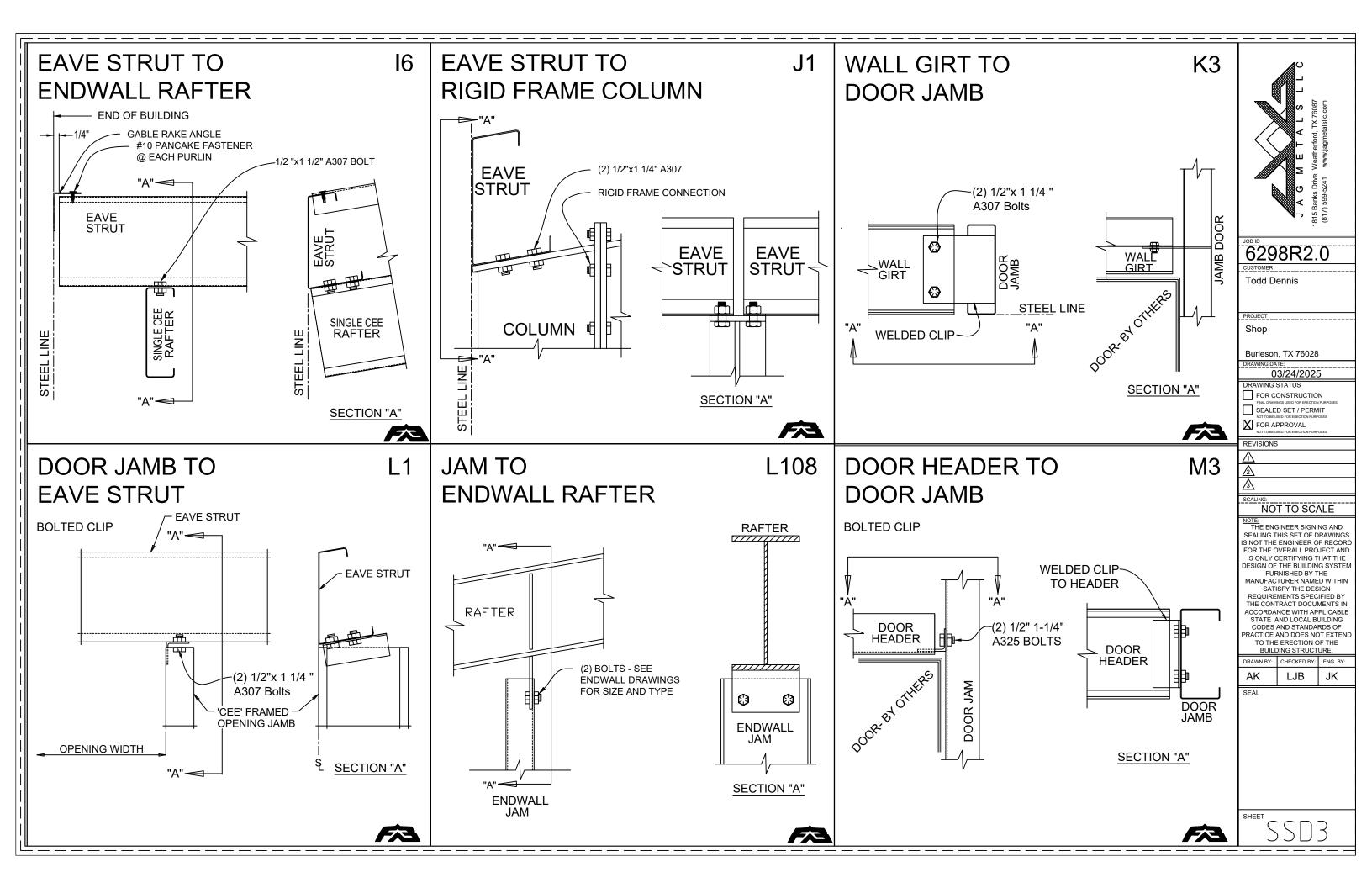


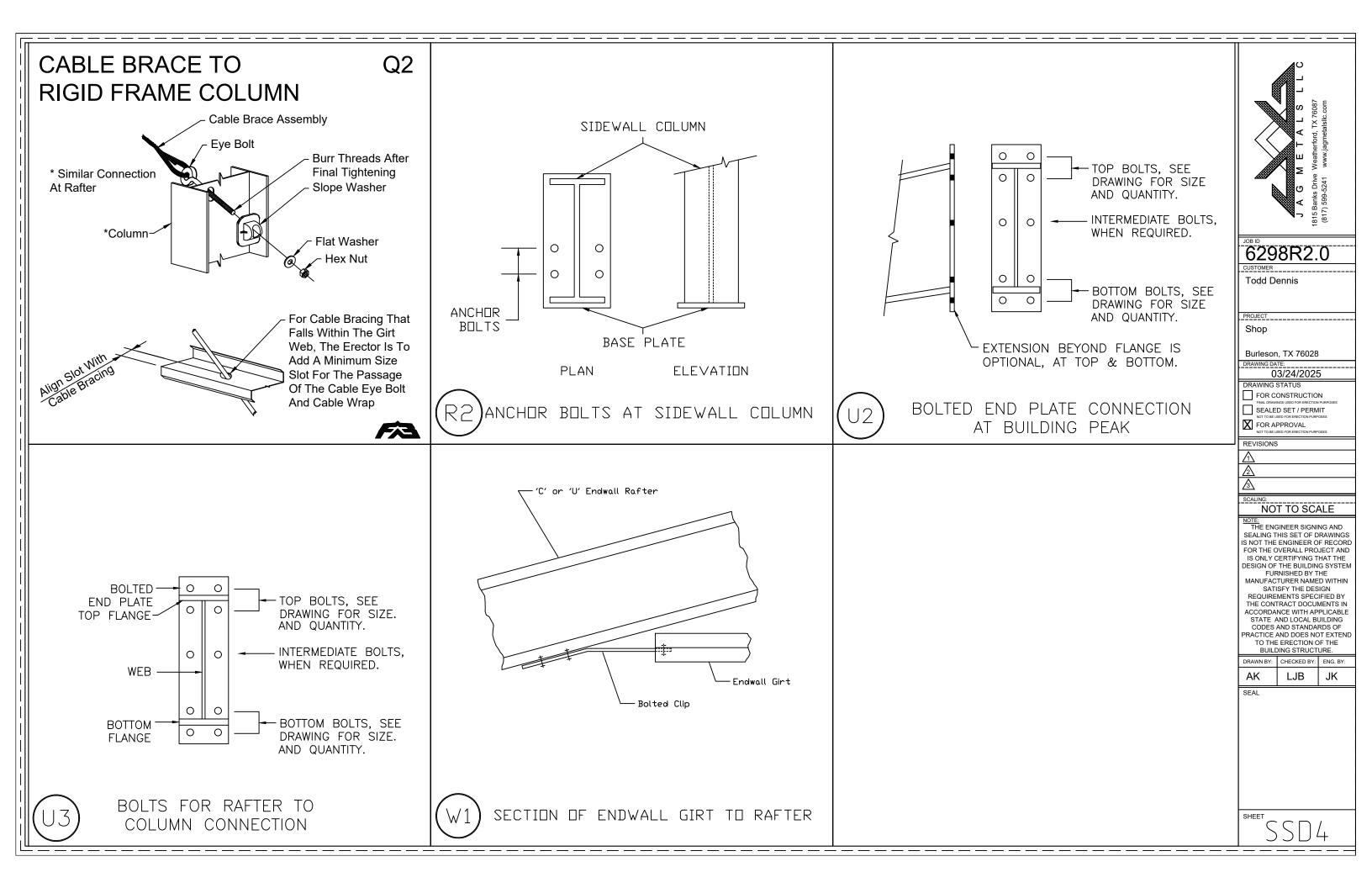
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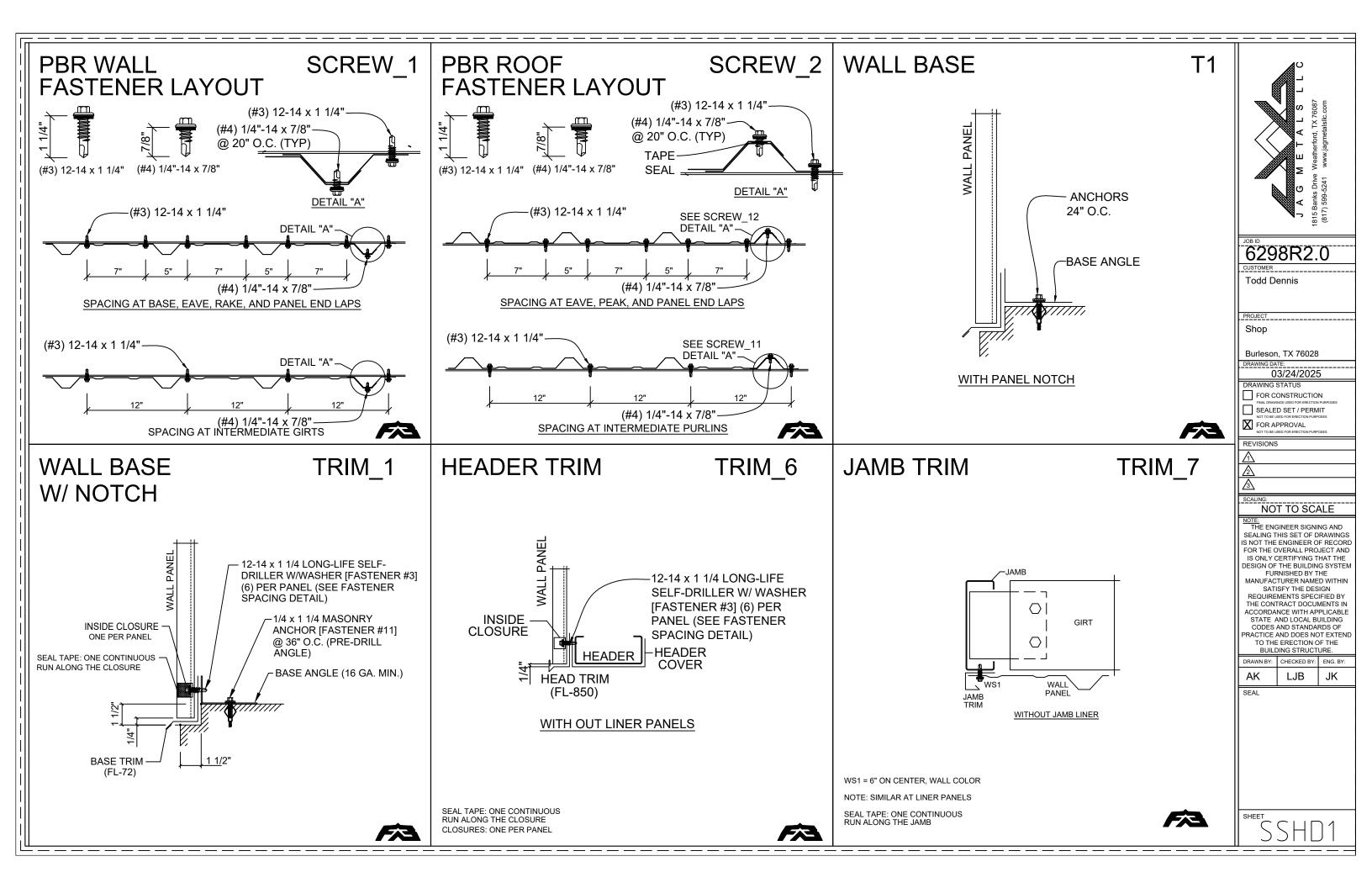
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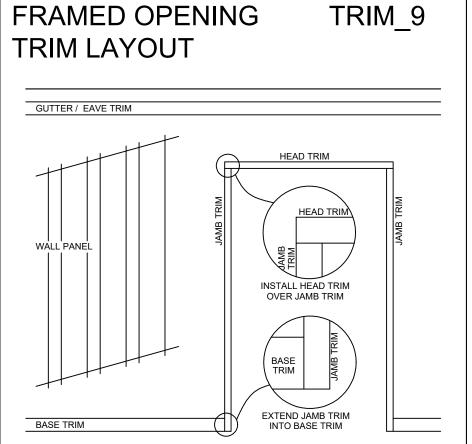
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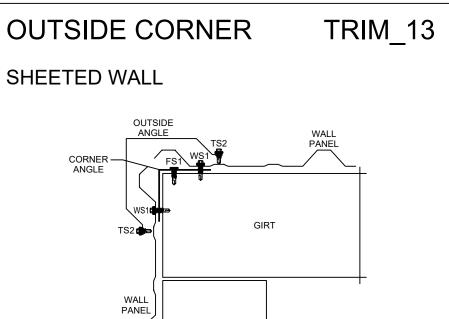
**SECTION "A"** 



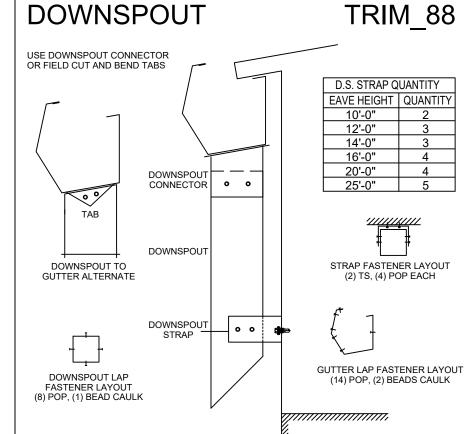




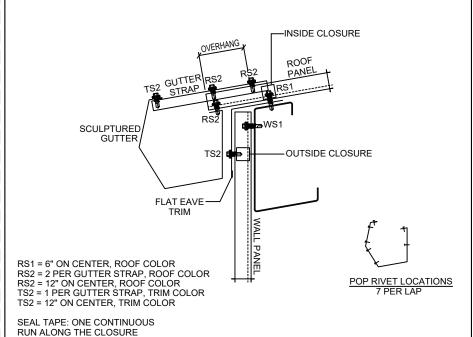




GIRT







GUTTER STRAP = 3" O.C.

CLOSURES: ONE PER PANEL

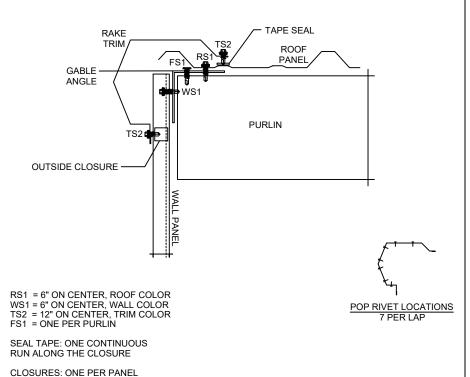
SHEETED GABLE

WS1 = 12" ON CENTER, WALL COLOR TS2 = 24" ON CENTER, TRIM COLOR FS1 = 1 PER CLIP

SEAL TAPE: TWO CONTINUOUS

RUNS ALONG THE CORNER

TRIM\_103



**RIDGE** 

SEAL TAPE: ONE CONTINUOUS RUN ALONG THE LAPS MAX ROOF PITCH: 4 1/2

ROOF PANEL RS1 = 6" ON CENTER, ROOF COLOR



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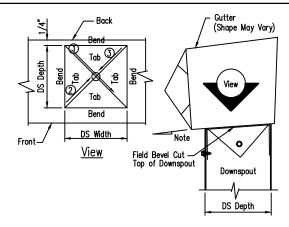
**TRIM 128** 

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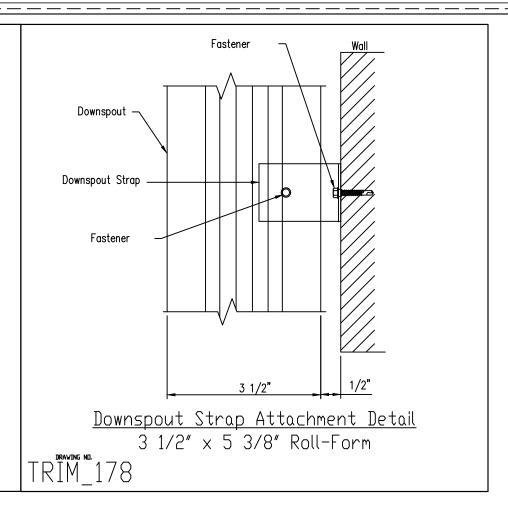
BUILDING STRUCTURE DRAWN BY: CHECKED BY: ENG. BY: ΑK LJB JK



### Downspout To Gutter Attachment Detail

- 1. Refer to the building erection drawings for the location and spacing of the downspouts.
- 2. Locate all downspouts over a major panel rib if possable.
- 3. Make a cardboard template of the downspout shape . Place the template on the bottom of the gutter and trace the outline. Remove the template and draw a line from corner to corner, forming an "X" pattern.
- 4. Drill a whole at the center of the "X". Using tin snips, cut along the lines of the X only. Do not cut along the outside lines of the downspout square.
- 5. Bend each triangular tab down toward the ground, 90 Degrees to the bottom of the gutter.
- Position the top of the downspout under the gutter. Make sure all four gutter tabs are on the inside of the downspout.
- 7. Install Fastener #4 1/4-14 X 7/8 Lap Tek self drilling fastener through the downspout into the gutter tab. Only the two sides and the front of the downspout will receive fasteners.

TRIM\_176





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DRAWN BY: CHECKED BY: ENG. BY:

AK LJB JK

BUILDING STRUCTURE.

SEAL

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